



A MATHEMATICAL MODEL FOR PREDICTING THE FLEXURAL STRENGTH CHARACTERISTICS OF CONCRETE MIXES MADE WITH GRANITE CHIPPINGS

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Abstract

Abakaliki, Ebonyi state of Nigeria produces most of the crushed-granite chippings used in the South-eastern part of the country. In this research work, these granite-chippings and fine aggregates from Amansea River in Anambra State of Nigeria were tested for their physical and mechanical properties based on BS 812: Parts 1&2:1975. Using these aggregates, sixty concrete beams of dimensions 600 mm X 150mm X 150 mm were made, cured and tested based on BS 1881:1983. Scheffe's (4, 2) lattice polynomial with regression equation was used to develop a mathematical model for predicting the flexural strength characteristics of concretes made with these aggregates. The mathematical model developed was $\hat{Y} = 4.28 x_1 + 4.42 x_2 + 3.4 x_3 + 2.71 x_4 + 0.2 x_1 x_2 + 0.04 x_1 x_3 - 0.14 x_1 x_4 - 0.08 x_2 x_3 - 0.3 x_2 x_4 + 1.22 x_3 x_4$. Finally, the student's t-test and the Fisher test were used to test the model's validity.

Keywords: Concrete, Flexural Strength, Scheffe, Granite-Chippings, Model

1. Introduction

1.1 Actual and Pseudo-Components

The requirement of the simplex that $x_1 + x_2 + x_3 + x_4 = 1$ makes it impossible to use the normal mix ratios such as 1:1:2, etc., at a given water/cement ratio. Hence, a transformation of the actual components (normal mix ratios) to meet this condition is unavoidable. The design matrix is shown in Table 1. $x^{(i)}_1, x^{(i)}_2, x^{(i)}_3$ and $x^{(i)}_4$ are the pseudo-components for the *ith* experimental points. For any actual component Z, the pseudo-component (x) is given by

$$X = AZ \quad (1)$$

Where A is the inverse of Z matrix and

$$Z = BX^T \quad (2)$$

Where B is the inverse of Z matrix and X^T is the transpose of the matrix.

Simplex is the structural representation of the line or planes joining the assumed positions of the constituent materials (atoms) of a mixture [1]. Scheffe [2] considered experiments with mixtures of which the property studied depended on the proportions of the components present but not on the quantity of the mixture. If a mixture has a total of q components and x_i be the proportion of the *ith* component in the mixture such that $x_i \geq 0$ ($i = 1, 2 \dots q$), then

$$x_1 + x_2 + x_3 + \dots + x_q = 1 \quad (3)$$

Scheffe described mixture properties by reduced polynomials obtainable from eqn (4):

$$\hat{Y} = b_0 + \sum b_i x_i + \sum b_{ij} x_i x_j + \sum b_{ijk} x_i x_j x_k + \sum b_{i_1 i_2 \dots i_n} x_{i_1} x_{i_2} \dots x_{i_n} \quad (4)$$

Where ($1 \leq i \leq q, 1 \leq j \leq q, 1 \leq i \leq k \leq q$) respectively and b is constant coefficient.

1.2 The Scheffe's (4, 2) Lattice Polynomial

Multiplying eqn. (3) by b_0 and multiplying the outcome by x_1, x_2, x_3 and x_4 in turn and substituting into eqn. (4), we have:

$$\hat{Y} = b_0 x_1 + b_0 x_2 + b_0 x_3 + b_0 x_4 + b_1 x_1 + b_2 x_2 + b_3 x_3 + b_4 x_4 + b_{12} x_1 x_2 + b_{13} x_1 x_3 + b_{14} x_1 x_4 + b_{23} x_2 x_3 + b_{24} x_2 x_4 + b_{34} x_3 x_4 + b_{11}(x_1 - x_1 x_2 - x_1 x_3 - x_1 x_4) + b_{22}(x_2 - x_1 x_2 - x_2 x_3 - x_2 x_4) + b_{33}(x_3 - x_1 x_3 - x_2 x_3 - x_3 x_4) + b_{44}(x_4 - x_1 x_4 - x_2 x_4 - x_3 x_4) \quad (5)$$

Re-arranging eqn. (5), we have

$$\hat{Y} = \sum \alpha_i x_i + \sum \alpha_{ij} x_i x_j \quad (6)$$

$$\alpha_{ij} = b_{ij} - b_{ii} + b_{jj} \text{ and } \alpha_i = b_0 + b_i + b_{ii} \quad (7)$$

Let the response function to the pure components (x_i) be denoted by y_i and the response to a 1:1 binary mixture of components i and j be y_{ij} . From eqn (6), it can be written that

$$\sum \alpha_i x_i = \sum y_i x_i \quad (8)$$

Where ($i = 1 \dots 4$)

Evaluating y_i , for instance gives:

$$y_i = \alpha_i \quad (9)$$

Also evaluating y_{ij} , gives in general the equations of the form

$$\alpha_{ij} = 4y_{ij} - 2y_i - 2y_j \quad (10)$$

For the Scheffe's (4, 2) lattice polynomial, that is eqn. (6) becomes:

$$\hat{Y} = y_1 x_1 + y_2 x_2 + y_3 x_3 + y_4 x_4 + (4y_{12} - 2y_1 - 2y_2) x_1 x_2 + (4y_{13} - 2y_1 - 2y_3) x_1 x_3 + (4y_{14} - 2y_1 - 2y_4) x_1 x_4 + (4y_{23} - 2y_2 - 2y_3) x_2 x_3 + (4y_{24} - 2y_2 - 2y_4) x_2 x_4 + (4y_{34} - 2y_3 - 2y_4) x_3 x_4 \quad (11)$$

1.2 The student's t-test

The unbiased estimate of the unknown variance S_Y^2 is given by Biyi [3]

$$S_Y^2 = \frac{\sum (y_i - \bar{Y})^2}{n - 1} \quad (12)$$

If $a_i = x_i (2x_i - 1)$, $a_{ij} = 4 x_i x_j$; for ($1 \leq i \leq q$) and ($1 \leq i \leq j \leq q$) respectively. Then,

$$\epsilon = \sum a_i^2 + \sum a_{ij}^2 \quad (13)$$

where ϵ is the error of the predicted values of the response. The t-test statistic is given by Biyi [3]:

$$t = \frac{\Delta Y}{S_Y} \frac{\sqrt{n}}{\sqrt{1 + \epsilon}} \quad (14)$$

where $\Delta Y = Y_0 - Y_t$; Y_0 = observed value, Y_t = theoretical value; n = number of replicate observations at every point; ϵ = as defined in eqn.(13).

1.3 The fisher's test

The Fishers-test statistic is given by

$$F = S_1^2 / S_2^2 \quad (15)$$

The values of S_1 (lower value) and S_2 (upper value) are calculated from eqn. (12).

2. Materials and method

2.1 Preparation, Curing and Testing of Beam Samples

The aggregates were sampled in accordance with the methods prescribed in BS 812: Part 1:1975 [4]. The test sieves were selected according to BS 410:1986 [5]. The water absorption, the apparent specific gravity and the bulk density of the coarse aggregates were determined following the procedures prescribed in BS 812: Part 2: 1975 [6]. The Los Angeles abrasion test was carried out in accordance with ASTM. Standard C131: 1976 [7]. The sieve analyses of the fine and coarse aggregate samples satisfied BS 882:1992 [8]. The sieving was performed by a sieve shaker. The water used in preparing the experimental samples satisfied the conditions prescribed in BS 3148:1980 [9]. The required concrete specimens were made in threes in accordance with the method specified in BS 1881: 109:1983 [10]. These specimens were cured for 28 days in accordance with BS 1881: Part 111: 1983 [11]. The testing was done in accordance with BS 1881: Part 118:1983 [12] using flexural testing machine.

2.2 Testing the Fit of the Quadratic Polynomials

The polynomial regression equation developed was tested to see if the model agreed with the actual experimental results. The null hypothesis (that there was agreement between the experimentally-observed data and the theoretically-obtained data) was denoted by H_0 and the alternative (that there was no agreement between these two) by H_1 .

3. Results and discussion

3.1 Physical and Mechanical Properties of Aggregates

The maximum aggregate size for the granite chipping was 20 mm and 2mm for the fine sand. The granite chippings had water absorption of 2.7%, moisture content of 44.2%, apparent specific gravity of 2.26, Los

Angeles abrasion value of 22% and bulk density of 2072.4 kg/m³.

3.2 The Regression Equation for the Flexural Strength Tests Results

Applying the responses (average flexural strengths) in determining the coefficients of the (4, 2) lattice polynomial to eqns. (9) and (10), we had $\alpha_1= 4.28$, $\alpha_2= 4.42$, $\alpha_3 =3.4$, $\alpha_4=2.71$, $\alpha_{12}=0.2$, $\alpha_{13} = 0.04$, $\alpha_{14}=- 0.14$, $\alpha_{23}= - 0.08$, $\alpha_{24}= -0.3$, $\alpha_{34}= 1.22$. Thus, from

eqn.(11): $\hat{Y} = 4.28 x_1+ 4.42 x_2+ 3.4 x_3+ 2.71 x_4+ 0.2 x_1 x_2+0.04 x_1 x_3 - 0.14 x_1 x_4 - 0.08 x_2 x_3 - 0.3 x_2 x_4+ 1.22 x_3 x_4$. This is the mathematical model for the response prediction of the flexural strength characteristics of the granite chippings concrete, based on Scheffe's (4, 2) polynomial. \hat{Y} represents the flexural strength of the concrete.

Table 1 Design Matrix for Scheffe's (4, 2) Lattice Polynomial

Legend: z₁= water/cement ratio; z₂=Cement; z₃=Fine aggregate; z₄=Coarse aggregate

Pseudo-components					Actual components			
S/N	x ₁	x ₂	x ₃	x ₄	z ₁	z ₂	z ₃	z ₄
1	1	0	0	0	0.6	1	1.5	2
2	0	1	0	0	0.5	1	1	2
3	0	0	1	0	0.55	1	2	5
4	0	0	0	1	0.65	1	3	6
5	½	½	0	0	0.55	1	1.25	2
6	½	0	½	0	0.575	1	1.75	3.5
7	½	0	0	½	0.625	1	2.25	4
8	0	½	½	0	0.525	1	1.5	3.5
9	0	½	0	½	0.575	1	2	4
10	0	0	½	½	0.6	1	2.5	5.5
Control								
11	½	¼	¼	0	0.5625	1	1.5	2.75
12	½	0	¼	¼	0.6	1	2.0	3.75
13	0	½	¼	¼	0.55	1	1.75	3.75
14	¼	¼	¼	¼	0.575	1	1.875	3.75
15	¾	¼	0	0	0.575	1	1.375	2
16	¾	0	¼	0	0.5875	1	1.625	2.75
17	¾	0	0	¼	0.6125	1	1.875	3.0
18	0	¾	¼	0	0.5125	1	1.25	2.75
19	0	¾	0	¼	0.5375	1	1.5	3.0
20	0	0	¾	¼	0.5850	1	2.25	5.25

Table 2: Flexural Strength Tests Results and Sample Variances, S_i², for Crushed –Granite Concrete, based on Scheffe's (4, 2) Simplex Lattices

S/NO	Replication	Responses y _i (N/mm ²)	Response symbol	Σy _i	Σy _i ²	ȳ	(Σy _i) ²	S _i ²
1	1A	4.15	y ₁	12.84	54.98	4.28	164.87	0.012
	1B	4.35						
	1C	4.34						
2	2A	4.30	y ₂	13.26	58.64	4.42	175.83	0.015
	2B	4.56						
	2C	4.40						

S/NO	Replication	Responses y_i (N/mm ²)	Response symbol	Σy_i	Σy_i^2	\bar{y}	$(\Sigma y_i)^2$	S_i^2
3	3A	3.00	y_3	10.2	34.97	3.4	104.04	0.145
	3B	3.45						
	3C	3.75						
4	4A	2.68	y_4	8.13	22.04	2.71	66.10	0.003
	4B	2.68						
	4C	2.77						
5	5A	4.60	y_{12}	13.2	58.15	4.4	174.24	0.035
	5B	4.25						
	5C	4.35						
6	6A	3.82	y_{13}	11.55	44.48	3.85	133.40	0.007
	6B	3.95						
	6C	3.78						
7	7A	3.50	y_{14}	10.38	35.97	3.46	107.74	0.028
	7B	3.60						
	7C	3.28						
8	8A	3.80	y_{23}	11.67	45.45	3.89	136.19	0.027
	8B	3.79						
	8C	4.08						
9	9A	3.44	y_{24}	10.47	36.55	3.49	109.62	0.005
	9B	3.58						
	9C	3.45						
10	10A	3.40	y_{34}	10.08	33.94	3.36	101.61	0.035
	10B	3.52						
	10C	3.16						
CONTROL								
11	11A	4.22	C_1	12.63	53.21	4.21	159.52	0.018
	11B	4.35						
	11C	4.06						
12	12A	3.82	C_2	10.8	39.09	3.6	116.64	0.105
	12B	3.75						
	12C	3.23						
13	13A	3.85	C_3	11.61	44.93	3.87	134.79	0.00
	13B	3.90						
	13C	3.86						
14	14A	3.95	C_4	11.49	44.03	3.83	132.02	0.012
	14B	3.78						
	14C	3.76						
15	15A	4.00	C_5	12.99	56.41	4.33	168.74	0.082
	15B	4.50						
	15C	4.49						
16	16A	4.10	C_6	11.7	45.70	3.9	136.89	0.035
	16B	3.85						
	16C	3.75						
17	17A	3.90	C_7	11.61	45.09	3.87	134.79	0.08
	17B	3.57						
	17C	4.14						
18	18A	4.20	C_8	13.2	58.32	4.4	174.24	0.12
	18B	4.80						
	18C	4.20						
19	19A	4.10	C_9	11.7	45.74	3.9	136.89	0.055
	19B	3.95						
	19C	3.65						

S/NO	Replication	Responses y_i (N/mm ²)	Response symbol	Σy_i	Σy_i^2	\bar{y}	$(\Sigma y_i)^2$	S_i^2
20	20A	3.25	C ₁₀	10.32	35.56	3.44	106.50	0.03
	20B	3.60						
	20C	3.47						

Table 3a: Regression Analysis of the Flexural Strength Tests Results

Regression Statistics	
Multiple R	0.99974617
R Square	0.9994924
Adjusted R Square	0.83257193
Standard Error	0.11475943
Observations	10

Table 3b: Analysis of variance

ANOVA					
	df	SS	MS	F	Significance F
Regression	4	155.5902816	38.89757	2953.559	1.28792E-08
Residual	6	0.079018365	0.01317		
Total	10	155.6693			

Table 3c: Regression Statistics

REGRESSION STATISTICS						
	Coefficients	Standard Error	t Stat	P-value	Lower 95%	Upper 95%
Intercept	0	#N/A	#N/A	#N/A	#N/A	#N/A
x ₁	4.1833	0.0844	49.541	4.54E-09	3.9766	4.3899
x ₂	4.5405	0.1056	42.98	1.06E-08	4.2820	4.7989
x ₃	3.7303	0.1546	24.12	3.33E-07	3.3519	4.1086
x ₄	2.5525	0.2679	9.52	7.63E-05	1.8969	3.2080

Legend df = degree of freedom, SS = sum of squares, MS = mean of squares, F = F-statistic, #N/A = insignificant value, ANOVA = analysis of variance.

3.3 Regression Analysis of the Flexural Strength Tests Results for the granite-chippings Concrete

Table 3 shows the summary output of the regression analysis of the flexural strength tests results of the granite-chippings concrete. The coefficient of determination, $r^2 = 0.9994$ shows a very strong relationship between the independent variables (x_1, x_2, x_3, x_4) and the dependent variable, \hat{Y} . Since the F-observed value of 2953.559 is very high; it is extremely unlikely that an F value this high occurred by chance. From the Student's t distribution table, t critical is 3.69. The absolute values of the t stat are greater than this t critical. This shows that all the variables used in the regression equation are useful in predicting the response. The P-values being very small means that the experimentally-obtained values and the predicted values of \hat{Y}

have variances that are not significantly different. Thus, the regression equation for the prediction of the flexural strength characteristics of the granite-chippings concrete is valid.

3.3 Fit of the Polynomial

The polynomial regression equation developed i.e., $\hat{Y} = 4.28 x_1 + 4.42 x_2 + 3.4 x_3 + 2.71 x_4 + 0.2 x_1 x_2 + 0.04 x_1 x_3 - 0.14 x_1 x_4 - 0.08 x_2 x_3 - 0.3 x_2 x_4 + 1.22 x_3 x_4$, was tested to see if the model agreed with the actual experimental results. There was no significant difference between the experimental and the theoretically expected results. The null hypothesis, H_0 was satisfied.

3.4 t-value from table

The t-student's test had a significance level, $\alpha = 0.05$ and $t_{\alpha/2(ve)} = t_{0.005(9)} = 3.69$. This was

greater than any of the t values calculated in table 4. Therefore, the regression equation for the crushed granite chippings concrete was adequate.

3.5 F-statistic analysis

Table 5 shows the F - statistic for the controlled points. The sample variances S_1^2 and S_2^2 for the two sets of data were not significantly different. It implied that the

error(s) from experimental procedure were similar and that the sample variances tested were estimates of the same population variance. Based on eqn. (12), we had that $S_K^2 = 0.82705/9 = 0.0919$, $S_E^2 = 0.604959/9 = 0.06722$ & $F = 0.0919/0.06722 = 1.367$. From Fisher's table, $F_{0.95(9,9)} = 3.3$, hence the regression equation for the flexural strength of the crushed-granite concrete was adequate.

Table 4: t -Statistic for the controlled Points, granite-chippings concrete flexural test, based on Scheffe's (4, 2) polynomial

Response symbol	$Y_K(N/mm^2)$	$Y_E(N/mm^2)$	$Y_K - \check{Y}_K$	$Y_E - \check{Y}_E$	$(Y_K - \check{Y}_K)^2$	$(Y_E - \check{Y}_E)^2$
C ₁	4.21	4.12	0.275	0.19975	0.075625	0.0399
C ₂	3.6	3.73125	-0.335	-0.189	0.112225	0.035721
C ₃	3.87	3.76625	-0.065	-0.154	0.004225	0.023716
C ₄	3.83	3.76125	-0.105	-0.159	0.011025	0.025281
C ₅	4.33	4.3525	0.395	0.43225	0.156025	0.18684
C ₆	3.9	4.0675	-0.035	0.14725	0.001225	0.021683
C ₇	3.87	3.86125	-0.065	-0.059	0.004225	0.003481
C ₈	4.4	4.15	0.465	0.22975	0.216225	0.052785
C ₉	3.9	3.93625	-0.035	0.016	0.001225	0.000256
C ₁₀	3.44	3.45625	-0.495	-0.464	0.245025	0.215296
Σ	39.35	39.2025			0.82705	0.604959

Legend: C_i =response; $a_i = x_i (2x_i - 1)$; $a_{ij} = 4 x_i x_j$; $\epsilon = \Sigma a_i^2 + \Sigma a_{ij}^2$; \check{y} = experimentally-observed value; \hat{Y} = theoretical value; t = t-test statistic.

Table 5: F -statistic for the controlled points, granite-chipping concrete flexural test, based on Scheffe's (4, 2) polynomial

Table 5a: Response symbol for C₁: $\epsilon = 0.6093$, $\check{Y} = 4.21N/mm^2$, $\hat{Y} = 4.12N/mm^2$ and $t = 0.456923$

i	j	a_i	a_{ij}	a_i^2	a_{ij}^2
1	2	0	0.5	0	0.25
1	3	0	0.5	0	0.25
1	4	0	0	0	0
2	3	-0.12	0.25	0.0156	0.0625
2	4	-0.12	0	0.0156	0
3	4	-0.12	0	0.0156	0
4	—	0	—	0	—
			Σ	0.0468	0.5625

Table YY: Response symbol for C₂-C₁₀

RESPONSE SYMBOL	ϵ	\check{Y} (N/mm ²)	\hat{Y} (N/mm ²)	t
C ₂	0.4842	3.6	3.73125	-0.72251
C ₃	0.7343	3.87	3.76625	0.488766
C ₄	0.5939	3.83	3.76125	0.35241
C ₅	0.2893	4.33	4.3525	-0.14258
C ₆	0.8593	3.9	4.0675	-0.73604
C ₇	0.5937	3.87	3.86125	0.044858
C ₈	0.4833	4.4	4.15	1.377045
C ₉	0.6405	3.9	3.93625	-0.18054
C ₁₀	0.4697	3.44	3.45625	-0.09034

Legend: $\check{Y} = \Sigma y/n$ where y is the response and n, the number of observed data (responses), Y_k is the experimental value (response), Y_E is the expected or theoretically calculated value (response)

4. Conclusion

The strengths (responses) of concrete were a function of the proportions of its ingredients: water, cement, fine aggregate and coarse aggregates. Since the predicted strengths by the model were in total agreement with the corresponding experimentally -observed values, the null hypothesis was satisfied. This meant that the model equation was valid.

References

1. Jackson, N. and Dhir, R. K. *Civil Engineering Material*, Macmillan ELBS, Hampshire RG21 2XS, England, 1988.
2. Scheffe, H., *Experiments with mixtures*, Royal Statistical Society Journal, Ser. B, Vol. 20, 1958, pp340-360.
3. Biyi, A., *Introductory Statistics*, Abiprint & Pak Ltd., Ibadan, 1975.
4. British Standard 812: Part 1 *Sampling, shape, size and classification*. Methods for sampling and testing of mineral aggregates, sands and fillers. British Standards Institution Publication, London, 1975.
5. British Standard 410 *Specification for test sieves*. British Standards Institution Publication, London, 1986.
6. British Standard 812: Part 2 *Methods for sampling and testing of mineral aggregates, sands and fillers*. *Physical properties*. British Standards Institution Publication, London, 1975.
7. ASTM. Standard C 131 *Tests for Resistance to Abrasion of Small Size Coarse Aggregate by Use of the Los Angeles Machine*. American Society for Testing and Materials Publication, New York, 1976.
8. British Standard 882 *Specification for aggregates from natural sources for concrete*. British Standards Institution Publication, London, 1992.
9. British Standard 3148 *Tests for water for making concrete*. British Standards Institution Publication, London, 1980.
10. British Standard 1881: Part 109 *Method for making test beams from fresh concrete*. British Standards Institution Publication, London, 1983.
11. British Standard 1881: Part 111 *Method of normal curing of test specimens (20 °C)*. British Standards Institution Publication, London, 1983.
12. British Standard 1881: Part 118. *Method for determination of flexural strength*. British Standards Institution Publication, London, 1983.